



Prepared for

Santee Cooper
One Riverwood Drive
Moncks Corner, SC 29461

INFLOW DESIGN FLOOD CONTROL SYSTEM PLAN SLURRY POND 3 AND 4 WINYAH GENERATING STATION

Prepared by

Geosyntec 
consultants

1300 South Mint Street, Suite 300
Charlotte, NC 28203

Project Number: 26009458

September 2026

INTRODUCTION

Winyah Generating Station (WGS or the Site) is a 1,260-megawatt coal-fired electric generating facility owned and operated by the South Carolina Public Service Authority (Santee Cooper). The Site is situated between Pennyroyal and Turkey Creeks and is located in Georgetown, South Carolina. Historically, coal combustion residuals (CCR) generated at WGS have been managed in existing CCR surface impoundments.

In response to the CCR Rule (40 Code of Federal Regulations (CFR) Part 257 Subpart D), Santee Cooper retained Geosyntec Consultants, Inc. (Geosyntec) to prepare documentation for existing surface impoundments at WGS. Pursuant to Section 257.82(c) of the CCR Rule, Geosyntec prepared this Inflow Design Flood Control System Plan for Slurry Pond 3 and 4 (Slurry Pond) at WGS.

Section 257.82(a) of the CCR Rule states that “*The owner or operator of an existing or new CCR surface impoundment, legacy CCR surface impoundment, or any lateral expansion of a CCR surface impoundment must design, construct, operate, and maintain an inflow design flood control system as specified in paragraphs (a)(1) and (2) of this section.*” The Preamble to the CCR Rule provides guidance on the documentation that should be provided for the Inflow Design Flood Control System Plan.

Section 257.82(b) of the CCR Rule states that the “*discharge from the CCR unit must be handled in accordance with the surface water requirements under §257.3-3*”.

The inflow design flood control system for the Slurry Pond at the Site consists of maintaining the minimum operating freeboard for the surface impoundment. Justification and documentation of the adequacy of the inflow design flood control system is presented in the following sections.

The work presented in this report was performed under the direction of Ms. Sarah Herr, PE, of Geosyntec in accordance with Section 257.82(c).

SURFACE IMPOUNDMENT DESCRIPTION

The Slurry Pond, encompassing approximately 107.2 acres (ac), is situated near the northwest corner of the Site, directly adjacent to Pennyroyal Creek. The Slurry Pond is bounded by perimeter dikes which are approximately 30 feet (ft) in height to the north, 26 ft in height to the west, and 20 ft in height to the east (Bowman Consulting Group, 2025). The Slurry Pond is separated from the West Ash Pond by a divider dike which forms the southern boundary of the Slurry Pond. Under the CCR Rule, the West Ash Pond is an inactive landfill. **Figure 1** depicts the boundary and hydraulic features associated with the Slurry Pond.

Stormwater from the exterior of the Slurry Pond is collected in Detention Ponds No. 1 and No. 2 located along the outside perimeter of the Slurry Pond. These Detention Ponds were designed to manage the 25-year (yr), 24-hour (hr) storm event (Santee Cooper, 2004). Pump Station No. 1 receives stormwater from Detention Pond No. 1, as well as Cooling Tower blowdown, and discharges to the West Low Volume Waste Pond. Pump Station No. 2 receives stormwater from Detention Pond No. 2 and discharges to the Slurry Pond. However, for this analysis, Pump Stations No. 1 and No. 2 are assumed to be non-operational due to temporary loss of power during a large storm event.

A Floating Pump Station equipped with two Tsurumi GSZ-4-45-4 submersible pumps normally conveys water from the Slurry Pond directly to the West Low Volume Waste Pond. The capacity of these pumps operating in parallel is 3,100 gallons per minute (gpm) at the normal operating elevation of 19.6 ft National Geodetic Vertical Datum of 1929 (NGVD 29) when pumping directly to the West Low Volume Waste Pond. Piping is valved such that the Floating Pump Station may convey water to Pump Station No. 1. However, for this analysis, the Floating Pump Station is assumed to be non-operational due to temporary loss of power during a large storm event.

The West Ash Pond is no longer hydraulically connected to the Slurry Pond and is not included in this evaluation.

CATCHMENT AREA AND DESIGN STORM EVENT

The contributing watershed area for the Slurry Pond is 107.2 ac. The surface impoundment is surrounded on all sides by a raised perimeter dike, which limits the stormwater run-on to that generated within the footprint of the pond itself. A description

of the drainage area is included in the Hydrologic and Hydraulic Analysis, provided in **Appendix A**.

Since the Slurry Pond is classified as a high hazard potential surface impoundment (Geosyntec, 2026), the inflow design flood is the Probable Maximum Flood (PMF).

STORAGE CAPACITY

The available stormwater storage volume in the Slurry Pond between elevations 4 ft and 36 ft NGVD 29 was calculated by developing an area-volume curve based on topographic data (Bowman Consulting Group, 2025). The lowest available contour within the Slurry Pond is 4 ft NGVD 29. The minimum crest elevation of the Slurry Pond perimeter dikes is 36 ft NGVD 29. The surface area of each contour was measured and tabulated at each elevation. Next, the available surface water volume in each depth increment was calculated by averaging the surface area of the upper and lower contours and multiplying by the change in elevation between the contours. The cumulative storage volume of the Slurry Pond between these elevations is 1,256.5 acre-feet (ac-ft). The area-volume data are presented in **Appendix A**.

HYDROLOGIC AND HYDRAULIC ANALYSIS

Geosyntec performed a hydrologic and hydraulic analysis of the Slurry Pond. Stormwater runoff volumes and associated discharges to the Slurry Pond were modeled using *HydroCAD Version 10.20* software (HydroCAD, 2024). The model assumed that the Floating Pump Station, Pump Station No. 1, and Pump Station No. 2 are non-operational due to temporary loss of power during the PMF. **Appendix A** presents the Hydrologic and Hydraulic Analysis and documents assumptions, rainfall abstractions, and drainage areas.

ROUTING RESULTS

The *HydroCAD* model results are presented in **Appendix A**. The resulting peak water surface elevation at the Slurry Pond during the PMF based on the hydrologic and hydraulic analysis is shown in **Table 1**. The Slurry Pond will effectively contain the PMF and maintain a freeboard of 7.7 ft assuming the Slurry Pond is maintained at the normal operating elevation of 19.6 ft NGVD 29. Detailed results are presented in **Appendix A**.

The results from this analysis show that the inflow design flood control system adequately manages flow into the CCR surface impoundment during and following the inflow design flood as specified in Section 257.82(a) of the CCR Rule. Additionally, all discharges from the Slurry Pond are managed through National Pollutant Discharge Elimination System Outfall 002 during normal operating conditions or contained within the CCR surface impoundment during the inflow design flood, thus complying with Section 257.82(b) of the CCR Rule.

Table 1 – Peak Elevations and Freeboard

Event	<i>Slurry Pond</i>	
	<i>Elevation (ft) (NGVD 29)</i>	<i>Freeboard (ft)</i>
Normal Operating Condition	19.6	16.4
PMF Condition	28.3	7.7

CERTIFICATION

This Inflow Design Flood Control System Plan meets the requirements of Section 257.82 of the CCR Rule and was prepared in accordance with current practices and the standard of care exercised by scientists and engineers performing similar tasks in the field of civil engineering. No other warranty is provided in connection therewith. The contents of this report are based solely on the observations of the conditions observed by Geosyntec and information provided to Geosyntec by Santee Cooper. Consistent with applicable professional standards of care, our opinions and recommendations were based in part on data furnished by others. Although we were not able to independently verify such data, we did evaluate it to determine whether it was consistent with other information that we developed in the course of our performance of the scope of services.

Certified by:



Sarah Herr, PE
Licensed State of SC, PE No. 42330

REFERENCES

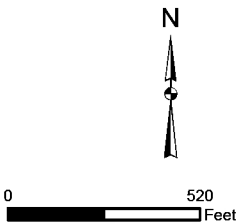
- Bowman Consulting Group. (2025). *Topographic Survey of Winyah Ash Pond West*.
- Geosyntec. (2026). *Hazard Potential Classification Assessment – Slurry Pond 3 and 4*.
- HydroCAD. (2024). *HydroCAD Stormwater Modeling System*. Version 10.20-3h.
- Santee Cooper. (2004). *Pump Station Design Report – Winyah Generating Station Redirect Drainage Project*.

FIGURES



Legend
Pond Boundary

Note:
1. Aerial imagery taken from Google Earth, dated April 2025.



WINYAH GENERATING STATION SLURRY POND 3 AND 4	
Geosyntec consultants	Figure 1
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APPENDIX A

Hydrologic and Hydraulic Analysis – Slurry Pond 3 and 4

Written by: A. Brand Date: 09/08/26 Reviewed by: S. Herr Date: 09/08/26
Client: Santee Cooper Project: WGS SP 3&4 H&H Project No.: 26009458 Phase No.: 04

**HYDROLOGIC AND HYDRAULIC ANALYSIS
WINYAH GENERATING STATION, SLURRY POND 3 AND 4,
GEORGETOWN, SOUTH CAROLINA**

1 PURPOSE AND BACKGROUND

Winyah Generating Station (WGS or the Site) is a 1,260-megawatt coal-fired electric generating facility owned and operated by the South Carolina Public Service Authority (Santee Cooper). The Site is situated between Pennyroyal and Turkey Creeks and is located in Georgetown, South Carolina. Historically, coal combustion residuals (CCR) generated at WGS have been managed in existing CCR surface impoundments.

The purpose of this computation package is to evaluate the hydraulic capacity of Slurry Pond 3 and 4 (Slurry Pond) to support spillway capacity assessments, static factor of safety analyses, and hazard rankings required by the United States Environmental Protection Agency's (USEPA's) Coal Combustion Residuals (CCR) Rule. The Slurry Pond is regulated by the CCR Rule as an existing CCR surface impoundment. Under the CCR Rule, a high hazard ranking classification is associated with the Probable Maximum Flood (PMF). Since the Slurry Pond is a high hazard surface impoundment, the PMF is analyzed herein.

The Slurry Pond, encompassing approximately 107.2 acres (ac), is situated near the northwest corner of the Site, directly adjacent to Pennyroyal Creek. The Slurry Pond is bounded by perimeter dikes which are approximately 30 feet (ft) in height to the north, 26 ft in height to the west, and 20 ft in height to the east (Bowman Consulting Group, 2025). The Slurry Pond is separated from the West Ash Pond by a divider dike which forms the southern boundary of the Slurry Pond. Under the CCR Rule, the West Ash Pond is an inactive landfill. **Figure 1** depicts the boundary and hydraulic features associated with the Slurry Pond.

Stormwater from the exterior of the Slurry Pond is collected in Detention Ponds No. 1 and No. 2 located along the outside perimeter of the Slurry Pond. These Detention Ponds were designed to manage the 25-year (yr), 24-hour (hr) storm event (Santee Cooper, 2004). Pump Station No. 1 receives stormwater from Detention Pond No. 1, as well as Cooling Tower blowdown, and discharges to the West Low Volume Waste Pond. Pump Station No. 2 receives stormwater from Detention Pond No. 2 and discharges to the Slurry Pond. However, for this analysis, Pump Stations No. 1 and No. 2 are assumed to be non-operational due to temporary loss of power during a large storm event.

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 Client: Santee Cooper Project: WGS SP 3&4 H&H Project No.: 26009458 Phase No.: 04

A Floating Pump Station equipped with two Tsurumi GSZ-4-45-4 submersible pumps normally conveys water from the Slurry Pond directly to the West Low Volume Waste Pond. The capacity of these pumps operating in parallel is 3,100 gallons per minute (gpm) at the normal operating elevation of 19.6 ft National Geodetic Vertical Datum of 1929 (NGVD 29) when pumping directly to the West Low Volume Waste Pond. Piping is valved such that the Floating Pump Station may convey water to Pump Station No. 1. However, for this analysis, the Floating Pump Station is assumed to be non-operational due to temporary loss of power during a large storm event.

The West Ash Pond is no longer hydraulically connected to the Slurry Pond and is not included in this evaluation.

2 METHODOLOGY AND INPUT PARAMETERS

Stormwater runoff volumes and associated discharges to the Slurry Pond were modeled using *HydroCAD Version 10.20* software (HydroCAD, 2024). *HydroCAD* utilizes frequency-based precipitation events, in conjunction with watershed properties, to calculate peak runoff by several accepted methods. The Soil Conservation Service (SCS) Technical Release 20 (TR-20) method was applied in *HydroCAD* to calculate stormwater runoff volumes (SCS, 1982).

The following parameters and assumptions were selected for calculating stormwater runoff volumes to the Slurry Pond.

Rainfall

The 72-hr duration precipitation event was used in this analysis. The rainfall depth corresponding to the 72-hr duration precipitation event for the PMF for the Site is 51 inches (in.) (NOAA, 1978). The design storm hydrograph was developed using SCS Type III rainfall distribution and was directly input to the *HydroCAD* model.

Drainage Areas and Curve Numbers

The contributing watershed area for the Slurry Pond is 107.2 ac. The area was delineated using the dike crests to correspond to the pond's direct drainage area. The pond was assigned a curve number (CN) based on guidance provided in Technical Release 55 (TR-55) (SCS, 1986) representing the type of ground cover in that area. The Slurry Pond was assumed to be approximately 94% ash (CN = 86) and 6% water (CN = 100) (Weighted CN = 87). The contributing watershed area and CN is summarized in **Table 1** and was directly input into the *HydroCAD* model.

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Times of Concentration

The time of concentration represents the time required for runoff to flow from the most hydraulically remote point of the drainage area to the point under investigation. The flow path from the most remote point within the Slurry Pond is characterized by sheet flow and shallow concentrated flow (shown in **Figure 2**). *HydroCAD* applied the Overton and Meadows formulation to calculate travel time for sheet flow for distances less than 300 ft (NRCS, 2010):

$$T_t = \frac{0.007(nL)^{0.8}}{P_{2-24}^{0.5}S^{0.4}}$$

where: T_t = travel time for overland sheet flow (hr);
 n = Manning's roughness coefficient for sheet flow (--);
 L = flow length (ft);
 P_{2-24} = 2-yr, 24-hr rainfall (in.); and
 S = slope of hydraulic grade line (or land slope) (ft/ft).

A Manning's roughness coefficient of 0.020 was used to represent sheet flow in the Slurry Pond. The sheet flow length was limited to 100 ft, because sheet flow beyond 100 ft typically transitions to shallow concentrated flow. The rainfall depth for the 2-yr, 24-hr frequency storm event is 4.38 in. (NOAA, 2021). The parameters used to model sheet flow within the Slurry Pond are shown in **Table 2**.

Shallow concentrated flow travel time was computed using the Upland Method (NRCS, 2010).

$$T_t = \frac{L}{V}$$

where: T_t = travel time (s);
 L = flow length (ft); and
 V = average velocity (ft/s).

The average velocity was computed using the following equation (NRCS, 2010).

$$V = K_v S^{0.5}$$

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where: V = average velocity (ft/s);
 K_v = velocity factor (ft/s); and
 S = slope of hydraulic grade line (or land slope) (ft/ft).

A velocity factor of 16.1 ft/s, representing flow across an unpaved surface, was used to calculate shallow concentrated flow travel time within the Slurry Pond. The parameters used to describe shallow concentrated flow within the Slurry Pond are presented in **Table 2**.

The resulting times of concentration for sheet flow and shallow concentrated flow are presented in **Table 2**.

Inflows

Stormwater inflow generated from the Slurry Pond is modeled as Subcatchment 1S. Stormwater inflow generated from Subcatchment 1S flows into Pond 1P (the Slurry Pond). The *HydroCAD* model routing diagram is provided in **Attachment A**.

Storage Capacities

Pond 1P models the available storage volume within the Slurry Pond. The available stormwater storage volume in the Slurry Pond between elevations 4 ft and 36 ft NGVD 29 was calculated by developing an area-volume curve based on topographic data (Bowman Consulting Group, 2025). The lowest available contour within the Slurry Pond is 4 ft NGVD 29. The minimum crest elevation of the Slurry Pond perimeter dikes is 36 ft NGVD 29. The surface area of each contour was measured and tabulated at each elevation. Next, the available surface water volume in each depth increment was calculated by averaging the surface area of the upper and lower contour and multiplying by the change in elevation between each contour. The cumulative storage volume of Slurry Pond between these elevations is 1,256.5 acre-feet (ac-ft). The area-volume data are presented in **Table 3**.

The Slurry Pond is maintained at a normal operating elevation of 19.6 ft NGVD 29 by the Floating Pump Station. As a result, the starting elevation of Pond 1P is set to 19.6 ft NGVD 29.

Outlet Structures

For this analysis, the Floating Pump Station is assumed to be non-operational due to temporary loss of power during a large storm event.

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3 RESULTS OF ANALYSES

The resulting peak water surface elevation and water volume for the PMF are shown in **Table 4**. The Slurry Pond will effectively contain the PMF and maintain a freeboard of 7.7 ft. This hydrologic and hydraulic analysis demonstrates that the Slurry Pond contains the PMF assuming the Slurry Pond is maintained at the normal operating elevation of 19.6 ft NGVD 29.

4 REFERENCES

- Bowman Consulting Group. (2025). *Topographic Survey of Winyah Ash Pond West*.
- HydroCAD. (2024). *HydroCAD Stormwater Modeling System*. Version 10.20-3h.
- NOAA. (1978). *Hydrometeorological Report No. 51: Probable Maximum Precipitation Estimates, United States East of the 105th Meridian*.
- NOAA. (2021). *Precipitation-Frequency Atlas of the United States*. Atlas 14, Volume 2, Version 3.0.
- NRCS. (2010). *National Engineering Handbook: Part 630 Hydrology*.
- Santee Cooper. (2004). *Pump Station Design Report – Winyah Generating Station Redirect Drainage Project*.
- SCS. (1982). *Technical Release Number 20 (TR-20)*. National Technical Information Service.
- SCS. (1986). *Technical Release Number 55 (TR-55)*. National Technical Information Service.

TABLES

Table 1. Land Use Conditions

Drainage Area ID	Land Use Description	CN	Area (acres)	Total Area (acres)	Weighted CN
Slurry Pond	CCR	86	100.8	107.2	87
	Water	100	6.4		

Table 2. Time of Concentration

Drainage Area ID	Time of Concentration - Sheet Flow Contribution						Time of Concentration - Shallow Concentrated Flow Contribution					Total Time of Concentration {Tc} (min)
	Surface Description	Manning's No. {n}	Flow Length {L} (ft)	2-Yr, 24-Hr Rainfall {I} (in.)	Land Slope {S} (ft/ft)	Time of Concentration from Sheet Flow {Tc, sheet} (min)	Surface Description	Velocity Factor (ft/s)	Flow Length {L} (ft)	Land Slope {S} (ft/ft)	Time of Concentration from Shallow Concentrated Flow {Tc, scf} (min)	
Slurry Pond	Smooth	0.020	100	4.38	0.0086	2.3	Unpaved	16.1	1,002	0.013	9.1	11.4

Table 3. Slurry Pond Stage Storage

Elevation (ft) (NGVD 29)	Area (ac)	Incremental Volume (ac-ft)	Cumulative Volume (ac-ft)
4	0.0	0.000	0.0
5	0.3	0.1	0.1
6	1.3	0.8	0.9
7	2.2	1.8	2.7
8	3.1	2.7	5.3
9	4.2	3.7	9.0
10	5.7	5.0	13.9
11	8.4	7.1	21.0
12	10.8	9.6	30.6
13	13.0	11.9	42.5
14	15.4	14.2	56.7
15	17.9	16.6	73.3
16	21.5	19.7	93.0
17	26.8	24.1	117.1
18	30.1	28.4	145.6
19	32.8	31.5	177.0
20	35.7	34.3	211.3
21	39.4	37.6	248.9
22	43.8	41.6	290.5
23	47.4	45.6	336.1
24	50.7	49.1	385.2
25	54.0	52.4	437.6
26	57.8	55.9	493.5
27	61.4	59.6	553.1
28	65.3	63.3	616.5
29	69.4	67.3	683.8
30	72.6	71.0	754.8
31	75.8	74.2	829.0
32	79.5	77.6	906.6
33	83.7	81.6	988.2
34	87.6	85.7	1,073.9
35	91.0	89.3	1,163.3
36	95.3	93.2	1,256.5

Table 4. H&H Calculation Results

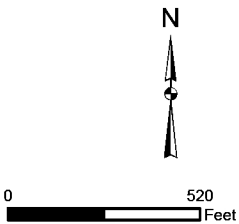
Pond ID	Storm Event	Normal Operating Elevation (ft) (NGVD 29)	Peak Water Surface Elevation (ft) (NGVD 29)	Water Volume (ac-ft)	Freeboard (ft)
Slurry Pond	PMF	19.6	28.3	440.0	7.7

FIGURES

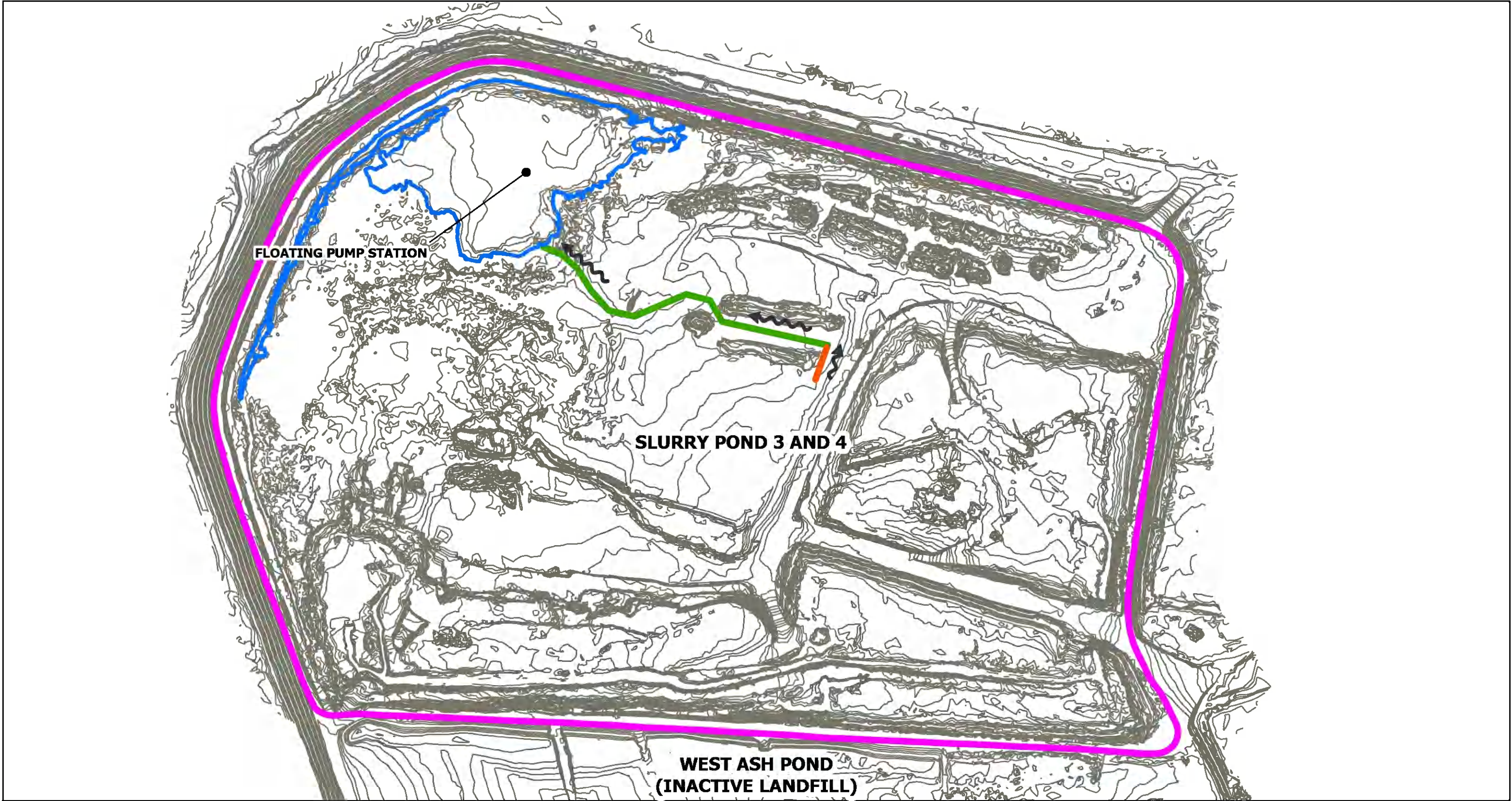


Legend
Pond Boundary

Note:
1. Aerial imagery taken from Google Earth, dated April 2025.



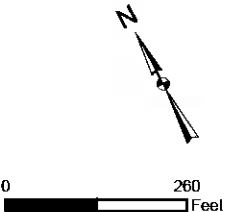
WINYAH GENERATING STATION SLURRY POND 3 AND 4	
Geosyntec consultants	Figure 1
26009458	September 2026



Legend

- Pond Boundary
- Normal Pool
- Shallow Concentrated Flow
- Sheet Flow
- ➔ Flow Direction

Note:
1. Topographic survey information collected by Bowman Consulting Group, dated 6-30-2025.



WINYAH GENERATING STATION
SLURRY POND 3 AND 4

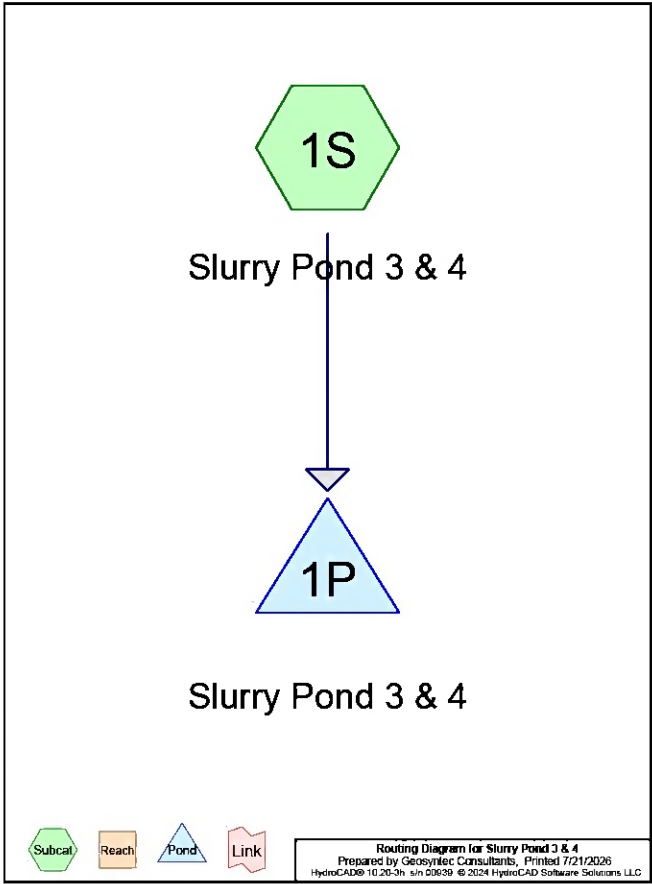
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Figure
2

ATTACHMENTS

ATTACHMENT A



Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
107.205	87	(1S)
107.205	87	TOTAL AREA

Rainfall Events Listing (selected events)

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	PMF (72-HR, 10-SQ MI)	Type III 24-hr		Scale	72.00	1	51.00	2

Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.000	HSG B	
0.000	HSG C	
0.000	HSG D	
107.205	Other	1S
107.205		TOTAL AREA

Ground Covers (all nodes)							
HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	0.000	0.000	107.205	107.205		1S
0.000	0.000	0.000	0.000	107.205	107.205	TOTAL AREA	

Time span=0.00-600.00 hrs, dt=0.050 hrs, 12001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Sim-Route method - Pond routing by Sim-Route method

Subcatchment 1S: Slurry Pond 3 & 4 Runoff Area=107.205 ac 0.00% Impervious Runoff Depth=49.25"
Flow Length=1,102' Tc=11.4 min CN=87 Runoff=1,930.76 cfs 439.984 af

Pond 1P: Slurry Pond 3 & 4 Peak Elev=28.31' Storage=637.580 af Inflow=1,930.76 cfs 439.984 af
Outflow=0.00 cfs 0.000 af

Total Runoff Area = 107.205 ac Runoff Volume = 439.984 af Average Runoff Depth = 49.25"
100.00% Pervious = 107.205 ac 0.00% Impervious = 0.000 ac

Summary for Subcatchment 1S: Slurry Pond 3 & 4

Runoff = 1,930.76 cfs @ 36.16 hrs, Volume= 439.984 af, Depth=49.25"
Routed to Pond 1P: Slurry Pond 3 & 4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-600.00 hrs, dt= 0.050 hrs
Type III 24-hr scaled to 72.00 hrs PMF (72-HR, 10-SQ MI) Rainfall=51.00"

Area (ac)	CN	Description			
107.205	87				
107.205		100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.3	100	0.0086	0.71		Sheet Flow, Sheet Flow n= 0.020 P2= 4.38"
9.1	1,002	0.0130	1.84		Shallow Concentrated Flow, Shallow Concentrated Flow Kv= 16.1 lps
11.4	1,102	Total			

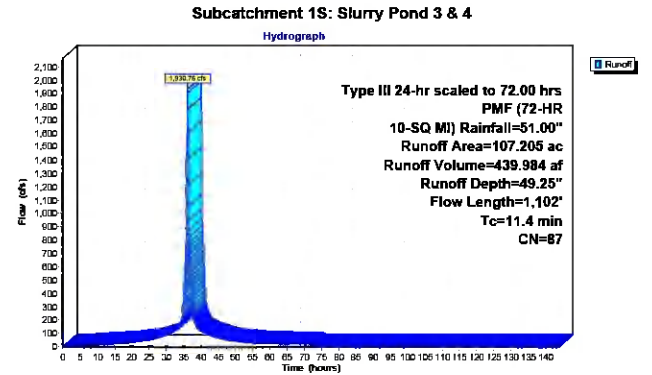
Summary for Pond 1P: Slurry Pond 3 & 4

Inflow Area = 107.205 ac, 0.00% Impervious, Inflow Depth = 49.25" for PMF (72-HR, 10-SQ MI) event
Inflow = 1,930.76 cfs @ 36.16 hrs, Volume= 439.984 af
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

Routing by Sim-Route method, Time Span= 0.00-600.00 hrs, dt= 0.050 hrs
Starting Elev= 19.60' Surf Area= 34.588 ac Storage= 197.596 af
Peak Elev= 28.31' @ 72.70 hrs Surf Area= 66.550 ac Storage= 637.580 af (439.984 af above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
Center-of-Mass del. time= (not calculated: no outflow)

Volume	Invert	Avail. Storage	Storage Description
#1	4.00'	1,256.476 af	Custom Stage Data (Prismatic), listed below
Elevation (feet)	Surf Area (acres)	Inc. Store (acre-feet)	Cum. Store (acre-feet)
4.00	0.008	0.000	0.000
5.00	0.264	0.136	0.136
6.00	1.261	0.762	0.899
7.00	2.243	1.752	2.650
8.00	3.121	2.682	5.333
9.00	4.184	3.653	8.985
10.00	5.743	4.963	13.949
11.00	8.389	7.066	21.015
12.00	10.764	9.577	30.591
13.00	13.027	11.895	42.487
14.00	15.381	14.204	56.691
15.00	17.904	16.643	73.333
16.00	21.466	19.685	93.018
17.00	26.788	24.127	117.145
18.00	30.057	28.423	145.568
19.00	32.844	31.450	177.018
20.00	35.750	34.297	211.315
21.00	39.448	37.599	248.914
22.00	43.780	41.614	290.528
23.00	47.449	45.615	336.143
24.00	50.731	49.090	385.232
25.00	54.025	52.378	437.610
26.00	57.817	55.921	493.531
27.00	61.420	59.619	553.150
28.00	65.254	63.337	616.487
29.00	69.389	67.321	683.808
30.00	72.592	70.990	754.799
31.00	75.828	74.210	829.009
32.00	79.452	77.640	906.649
33.00	83.748	81.600	988.249
34.00	87.643	85.695	1,073.944
35.00	91.036	89.339	1,163.284
36.00	95.348	93.192	1,256.476



Pond 1P: Slurry Pond 3 & 4

